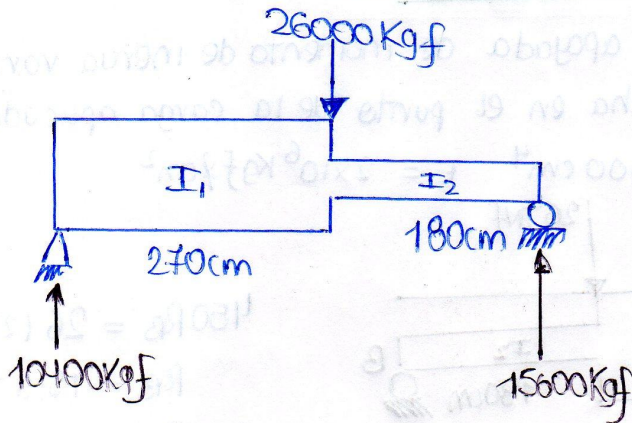


SINGULARIDAD



$$I_1 E y_1'' = 10400 x$$

$$I_1 E y_1' = 10400 x^2/2 + C_1$$

$$I_1 E y_1 = 10400 x^3/6 + C_1 x + C_2$$

$$I_2 E y_2'' = 10400 x_2 - 26000 < x_2 - 270 >$$

$$I_2 E y_2' = 10400 x_2^2/2 - 26000 < x_2 - 270 >^2/2 + C_3$$

$$I_2 E y_2 = 10400 x_2^3/6 - 26000 < x_2 - 270 >^3/6 + C_3 x_2 + C_4$$

✓ Para $x_1 = 270 \text{ cm}$ $x_2 = 270 \text{ cm}$ $y_1 = y_2$

$$10400 \left(\frac{270}{6} \right)^3 + 270 C_1 + C_2 = \left(10400 \left(\frac{270}{6} \right)^3 + 270 C_3 + C_4 \right) \left(\frac{16680}{12500} \right)$$

$$270 C_1 - 359.64 C_3 - 1.332 C_4 - 1.13269104 \times 10^{10} = 0$$

✓ Para $x_1 = 270 \text{ cm}$ $x_2 = 270 \text{ cm}$ $y_1' = y_2'$

$$10400 \left(\frac{270}{2} \right)^2 + C_1 = \frac{16680}{12500} \left(10400 \left(\frac{270}{2} \right)^2 + C_3 \right)$$

$$C_1 - 1.332 C_3 - 125854560 = 0$$

✓ Para $x_2 = 450 \text{ cm}$ $y_2 = 0$

$$0 = 10400 \left(\frac{450}{6} \right)^3 - 26000 \left(\frac{180}{6} \right)^3 + 450 C_3 + C_4$$

$$0 = 450 C_3 + C_4 + 1.32678 \times 10^{11}$$

$$C_1 = -317214144$$

$$C_3 = -332634162.2$$

$$C_4 = 1.700737297 \times 10^{10}$$

Deflexión $x = 270 \text{ cm}$

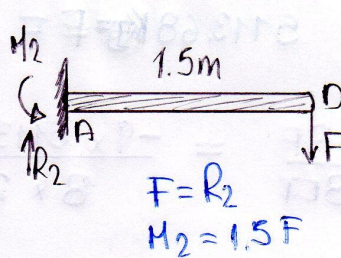
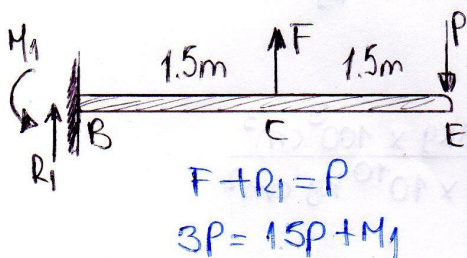
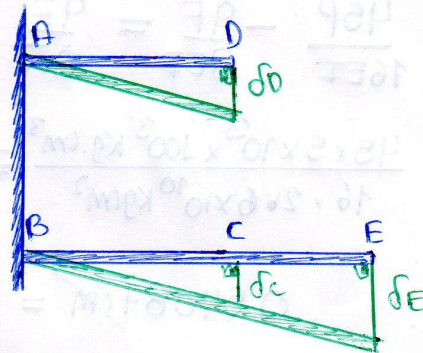
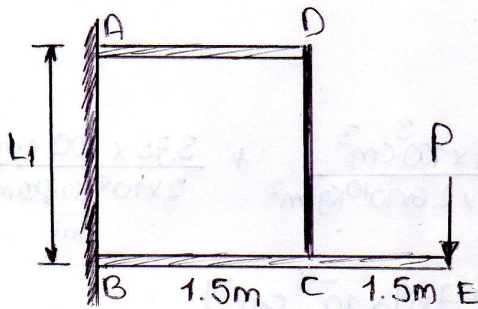
Giro $x = 270 \text{ cm}$

$$y_1 = -1.547466 \text{ cm}$$

$$\Theta = 1.857834 \times 10^{-3}$$

Singularidad - Superposición

Dos vigas en cantilever AD y BE de igual rigidez a la flexión $EI = 2.6 \times 10^{10} \text{ Kg}\cdot\text{cm}^2$ están conectadas por una varilla de acero DC para la cual $E_1 = 2 \times 10^6 \text{ Kg}\cdot\text{cm}^2$, $A_1 = 3 \text{ cm}^2$ y $L_1 = 3.75 \text{ m}$. Se pide hallar la flecha del voladizo en "D" debido a la fuerza $P = 5 \text{ TN}$.



$$EI y_1'' = R_1 x - M_1 + F \langle x - 1.5 \rangle$$

$$EI y_2'' = R_2 x - M_2$$

$$EI y_1' = \frac{R_1 x^2}{2} - M_1 x + \frac{F \langle x - 1.5 \rangle^2}{2} + C_1$$

$$EI y_2' = \frac{R_2 x^2}{2} - M_2 x + C_3$$

$$EI y_1 = \frac{R_1 x^3}{6} - \frac{M_1 x^2}{2} + \frac{F \langle x - 1.5 \rangle^3}{6} + C_1 x + C_2$$

$$EI y_2 = \frac{R_2 x^3}{6} - \frac{M_2 x^2}{2} + C_3 x + C_4$$

Para $x = 0$ $y_1 = 0$ $C_2 = 0$
 $x = 0$ $y_1' = 0$ $C_1 = 0$

Para $x = 0$ $y_2 = 0$ $C_4 = 0$

Para $x = 0$ $y_2' = 0$ $C_3 = 0$

Para $x = 1.5$ $y_1 = \delta_C$

Para $x = 1.5 \text{ m}$ $y_2 = \delta_D$

$$y_1 = \frac{9R_1}{16EI} - \frac{9M_1}{8EI}$$

$$y_2 = \frac{9R_2}{16EI} - \frac{9M_2}{8EI}$$

$$y_1 = \frac{9P}{16EI} - \frac{9F}{16EI} - \frac{27P}{8EI} + \frac{27F}{16EI}$$

$$y_2 = \frac{9F}{16EI} - \frac{27F}{16EI}$$

$$\delta_C = \frac{9F}{8EI} - \frac{45P}{16EI}$$

$$\delta_D = \frac{-9F}{8EI}$$

$$\delta_c = \frac{9F}{16EI} - \frac{45P}{16EI} \rightarrow \delta_c = \frac{45P}{16EI} - \frac{9F}{8EI} (\downarrow)$$

$$\delta_D = -\frac{9F}{8EI} \rightarrow \delta_D = \frac{9F}{8EI} (\downarrow)$$

$$\delta_c = \delta_D + \delta_{cable}$$

$$\frac{45P}{16EI} - \frac{9F}{8EI} = \frac{9F}{8EI} + \frac{FL}{AEI}$$

$$\frac{45 \times 5 \times 10^3 \times 100^3 \text{ Kg.cm}^3}{16 \times 2 \times 6 \times 10^{10} \text{ Kg.cm}^2} = F \left[\frac{9 \times 100^3 \text{ cm}^3}{8 \times 2.6 \times 10^{10} \text{ Kg.cm}^2} + \frac{3.75 \times 100 \text{ cm}}{2 \times 10^8 \frac{\text{Kg}}{\text{cm}^2} \times 3 \text{ cm}^2} \right]$$

$$0.54087 \text{ cm} = F \left[1.05769 \times 10^{-4} \frac{\text{cm}}{\text{Kg}} \right]$$

$$5113.68 \text{ Kg} = F$$

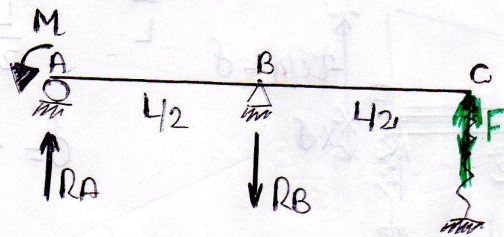
$$\delta_D = -\frac{9F}{8EI} = \frac{-9 \times 5113.68 \text{ Kg} \times 100^3 \text{ cm}^3}{8 \times 2.6 \times 10^{10} \text{ Kg.cm}^2}$$

$$\delta_D = 0.22127 \text{ cm} \quad \underline{\text{RPTA}}$$

Singularidad

La viga ABC tiene una rigidez a la flexión EI y una longitud L .

El extremo C esta unido a un resorte de constante K . determinar la fuerza en el resorte debido al momento aplicado en A.



$$\begin{aligned} M_A &= 0 & 2F + \frac{2M}{L} &= R_B \\ M_B &= 0 & F + \frac{2M}{L} &= R_A \end{aligned}$$

$$EI y'' = R_A x - M - R_B \left\langle x - \frac{L}{2} \right\rangle$$

$$EI y' = \frac{R_A x^2}{2} - Mx - \frac{R_B}{2} \left\langle x - \frac{L}{2} \right\rangle^2 + C_1$$

$$EI y = \frac{R_A x^3}{6} - \frac{Mx^2}{2} - \frac{R_B}{6} \left\langle x - \frac{L}{2} \right\rangle^3 + C_1 x + C_2$$

$$y = -\frac{FL^3}{12EI} + \frac{ML^2}{24EI} \quad \left(\downarrow \right) \Rightarrow x = \frac{F}{K} \downarrow$$

Para $x=0$ $C_2=0$

Para $x=\frac{L}{2}$ $y=0$

$$C_1 = \frac{ML}{4} - \frac{RAL^3}{24}$$

$$\frac{F}{K} + \frac{FL^3}{12EI} = \frac{ML^2}{24EI}$$

$$F \left[\frac{12EI + KL^3}{12KEI} \right] = \frac{ML^2}{24EI}$$

$$F = \frac{K ML^2}{24EI + 2KL^3}$$

Para $x=L$

$$EI y = \frac{RAL^3}{8} - \frac{RL^3}{48} - \frac{ML}{4}$$

$$EI y = \frac{FL^3}{8} + \frac{ML^2}{4} - \frac{FL^3}{24} - \frac{ML^2}{24} - \frac{ML^2}{4}$$

$$EI y = \frac{FL^3}{12} - \frac{ML^2}{24}$$

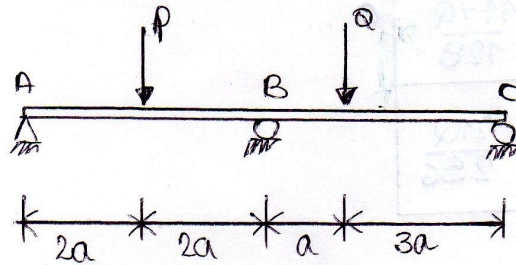
$$y = \frac{FL^3}{12EI} - \frac{ML^2}{24EI}$$

$$y = - \left[-\frac{FL^3}{12EI} + \frac{ML^2}{24EI} \right]$$

$$y = \frac{ML^2}{24EI} - \frac{FL^3}{12EI}$$

SINGULARIDAD

Para la viga ABC mostrada, donde EI es constante, determinar la relación entre las fuerzas P y Q, para la fuerza cortante en "c" sea siempre negativa.



$$\begin{aligned}
 EIY'' &= R_A x - P\langle x-2a \rangle + R_B \langle x-4a \rangle - Q\langle x-5a \rangle \\
 EIY' &= \frac{R_A x^2}{2} - \frac{P\langle x-2a \rangle^2}{2} + \frac{R_B \langle x-4a \rangle^2}{2} - \frac{Q\langle x-5a \rangle^2}{2} + C_1 \\
 EIY &= \frac{R_A x^3}{6} - \frac{P\langle x-2a \rangle^3}{6} + \frac{R_B \langle x-4a \rangle^3}{6} - \frac{Q\langle x-5a \rangle^3}{6} + C_1 x + C_2
 \end{aligned}$$

Para $x=0$ $y=0$ $C_2=0$

Para $x=4a$ $y=0$ $C_1=?$

$$0 = \frac{32R_A a^3}{3} - \frac{4P a^3}{3} + C_1(4a)$$

$$C_1 = \frac{P a^2}{3} - \frac{8R_A a^2}{3}$$

Para $x=8a$ $y=0$

$$0 = \frac{256R_A a^3}{3} - 36P a^3 + \frac{32R_B a^3}{3} - \frac{9Q a^3}{2} + \frac{8P a^3}{3} - \frac{64R_A a^3}{3}$$

$$0 = 64R_A + \frac{32R_B}{3} - \frac{100P}{3} - \frac{9Q}{2} \quad \dots (I)$$

ESTÁTICA

$$8R_A(a) + 4R_B(a) - 6Pa - 3Qa = 0$$

$$8R_A + 4R_B - 6P - 3Q = 0$$

$$8R_A + 32R_B - 48P - 24Q = 0 \quad \dots (II)$$

(II) - (I)

$$\frac{64R_B}{3} - \frac{44P}{3} - \frac{39Q}{2} = 0$$

$$R_B = \frac{11P}{16} + \frac{117Q}{128}$$
$$R_A = \frac{13P}{32} - \frac{21Q}{256}$$

ESTÁTICA

$$R_A + R_B + R_C = P + Q$$

$$R_C = \frac{43Q}{256} - \frac{3P}{32}$$

$$R_C > 0$$

$$\frac{43Q}{256} - \frac{3P}{32} > 0$$

$$\frac{43Q}{256} > \frac{3P}{32}$$

$$\frac{43}{24} > \frac{P}{Q} \quad \text{RPTA}$$

$$\frac{P}{8} - \frac{9Q}{8} - \frac{9Q}{8} + \frac{9Q}{8} = 0$$

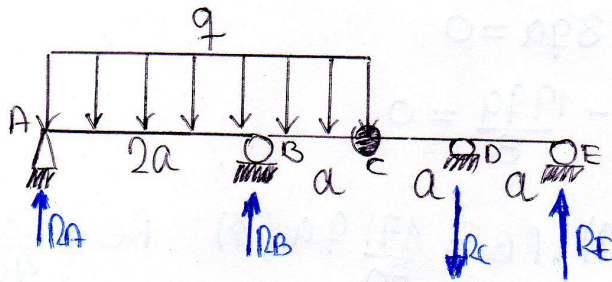
$$0 = P - 9Q - 9Q + 9Q$$

$$0 = P - 9Q - 9Q + 9Q$$

$$0 = P - 9Q - 9Q + 9Q$$

Singularidad

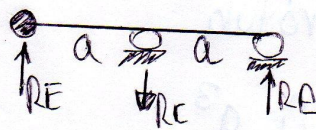
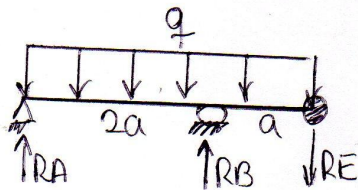
Conociendo $q, E, q, e \pm$; calcular la deflexión bajo la rótula



$$3R_A + R_B = 4.5qa$$

$$2R_E = R_C$$

$$R_A + R_B - R_E = 3qa$$



$$EIY_1'' = R_A x - \frac{q}{2} x^2 + R_B \langle x - 2a \rangle$$

$$EIY_1' = \frac{R_A x^2}{2} - \frac{q}{6} x^3 + \frac{R_B}{2} \langle x - 2a \rangle^2 + C_1$$

$$EIY_1 = \frac{R_A x^3}{6} - \frac{q}{24} x^4 + \frac{R_B}{6} \langle x - 2a \rangle^3 + C_1 x + C_2$$

Para $x=0$ $y=0$ $C_2=0$

Para $x=2a$ $y=0$ $C_1=?$

$$0 = \frac{8R_A}{6} a^3 - \frac{16qa^4}{24} + 2aC_1$$

$$C_1 = \frac{16qa^3}{48} - \frac{8R_A a^2}{12}$$

Para $x=3a$ $y_1 = y_2$

$$\frac{27R_A}{6} a^3 - \frac{81qa^4}{24} + \frac{R_B a^3}{6} + qa^4 - 2R_A a^3$$

$$EIY_1 = \frac{5R_A}{2} a^3 + \frac{R_B a^3}{6} - \frac{19}{8} qa^4$$

$$\frac{5}{2} R_A + \frac{R_B}{6} - \frac{2RE}{3} - \frac{19}{8} qa = 0$$

$$EIY_2'' = R_E x - R_C \langle x - a \rangle$$

$$EIY_2' = \frac{R_E x^2}{2} - \frac{R_C}{2} \langle x - a \rangle^2 + C_3$$

$$EIY_2 = \frac{R_E x^3}{6} - \frac{R_C}{6} \langle x - a \rangle^3 + C_3 x + C_4$$

Para $x=a$ $y=0$

$$0 = \frac{RE a^3}{6} + C_3 a + C_4$$

Para $x=2a$ $y=0$

$$0 = \frac{8RE}{6} a^3 - \frac{R_C}{6} a^3 + 2aC_3 + C_4$$

$$C_3 = -\frac{5RE a^2}{6}$$

$$C_4 = \frac{2RE a^3}{3}$$

Para $x=0$ $y_2 = y_1$

$$EIY_2 = C_4 = \frac{2RE a^3}{3}$$

$$3R_A + R_B - 4.5q_a = 0$$

$$R_A + R_B - R_E - 3q_a = 0$$

$$\frac{5R_A}{2} + \frac{R_B}{6} - \frac{2R_E}{3} - \frac{19q_a}{8} = 0$$

$$R_A = \frac{63}{80} q_a (\uparrow) \quad R_B = \frac{171}{80} q_a (\uparrow) \quad R_E = \frac{39}{40} q_a (\downarrow)$$

Deflexión rótula

$$\delta_c = \frac{2R_E}{3EI} a^3$$

$$\delta_c = \frac{+2}{3EI} \left(-\frac{39q_a}{40} \right) a^3$$

$$\delta_c = -\frac{q_a^4}{20EI}$$

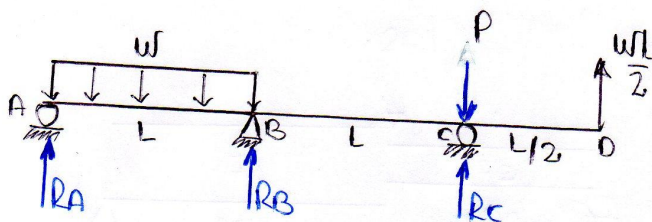
$$\delta_c = \frac{q_a^4}{20EI} (\downarrow)$$

Rpta.

Comprobado por
Área de momentos

Singularidad

Utilizando el método funciones de singularidad, calcular los momentos en los apoyos y las reacciones. dibujar los diagramas de DFC y DMF.



$$IEY'' = R_A x - \frac{wx^2}{2} + R_B \langle x-L \rangle + \frac{w \langle x-L \rangle^2}{2} - P \langle x-2L \rangle + R_C \langle x-2L \rangle$$

$$IEY' = \frac{R_A x^2}{2} - \frac{wx^3}{6} + \frac{R_B \langle x-L \rangle^2}{2} + \frac{w \langle x-L \rangle^3}{6} - \frac{P \langle x-2L \rangle^2}{2} + \frac{R_C \langle x-2L \rangle^2}{2} + C_1$$

$$IEY = \frac{R_A x^3}{6} - \frac{wx^4}{24} + \frac{R_B \langle x-L \rangle^3}{6} + \frac{w \langle x-L \rangle^4}{24} - \frac{P \langle x-2L \rangle^3}{6} + \frac{R_C \langle x-2L \rangle^3}{6} + C_1 x + C_2$$

Para $x=0$ $y=0$ $C_2 = 0$

Para $x=L$ $y=0$ $C_1 = \frac{wL^3}{24} - \frac{R_A L^2}{6}$

Para $x=2L$ $y=0$ $0 = 24R_A + 4R_B - 13wL$

$M_C = 0$ $0 = 2R_A + R_B - \frac{7}{4}wL$

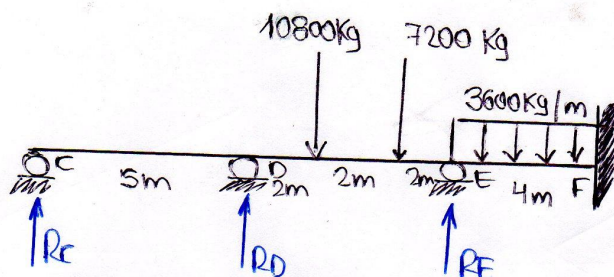
$\begin{matrix} 0 \\ 00 \end{matrix}$ $R_A = \frac{3wL}{8}$

$$R_B = wL$$

$$R_C = P - \frac{7wL}{8}$$

Singularidad

Para la viga cargada como se indica, determine los DFC y DMF. Aplique el método de funciones de singularidad. $EI = \text{cte}$



$$IEY'' = R_c x + R_D \langle x-5 \rangle - 10800 \langle x-7 \rangle - 7200 \langle x-9 \rangle + R_E \langle x-11 \rangle - 3600 \frac{\langle x-13 \rangle^2}{2}$$

$$IEY' = \frac{R_c x^2}{2} + \frac{R_D}{2} \langle x-5 \rangle^2 - 5400 \langle x-7 \rangle^2 - 3600 \langle x-9 \rangle^2 + \frac{R_E}{2} \langle x-11 \rangle^2 - 600 \langle x-13 \rangle^3 + C_1$$

$$IEY = \frac{R_c x^3}{6} + \frac{R_D}{6} \langle x-5 \rangle^3 - 1800 \langle x-7 \rangle^3 - 1200 \langle x-9 \rangle^3 + \frac{R_E}{6} \langle x-11 \rangle^3 - 150 \langle x-13 \rangle^4 + C_1 x + C_2$$

Para $x=0$ $y=0$ $C_2=0$

Para $x=5$ $y=0$ $C_1 = -\frac{25}{6} R_c$

Para $x=11$ $y=0$ $0 = 44R_c + 9R_D - 31200$

Para $x=15$ $y'=0$ $0 = 325R_c + 150R_D + 24R_E - 1940000$

Para $x=15$ $y=0$ $0 = 375R_c + 125R_D + 8R_E - 887400$

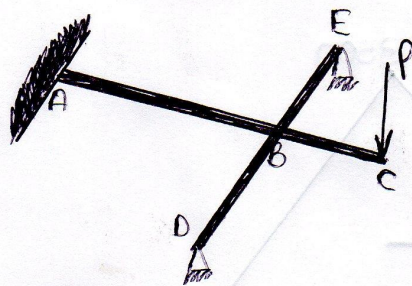
$$R_c = -1474 \text{ Kg}$$

$$R_D = +10672.89$$

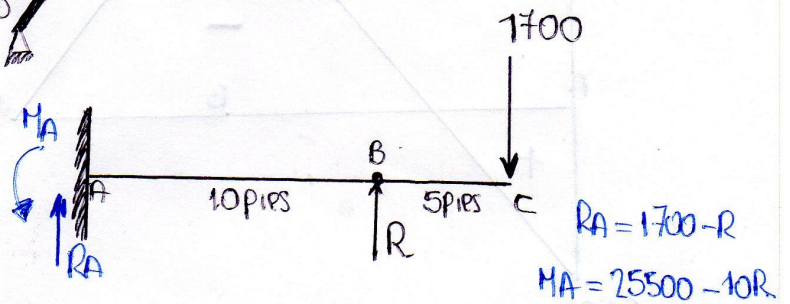
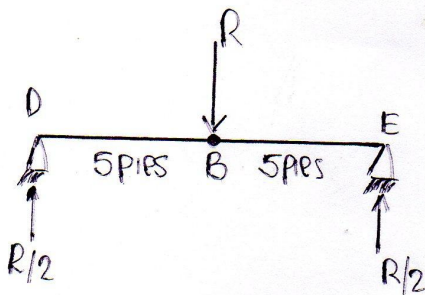
$$R_E = +13254.86$$

Singularidad

La viga ABC esta empotrada en "A" y se apoya en el punto medio de la viga DE. La distancia de A a B es de 10 pies, la distancia de B a C es 5 pies y la longitud de la viga DE es 10 pies, ambas vigas tienen la misma rigidez (EI). dibujar los diagramas de fuerza cortante y momento flector de la viga ABC, sabiendo que $P = 1700$ Libras.



AC Sobre DB



$$R_A = 1700 - R$$

$$M_A = 25500 - 10R$$

$$EIY_1'' = \frac{R}{2}x - R\langle x-5 \rangle$$

$$EIY_1' = \frac{R}{2} \cdot \frac{x^2}{2} - R \frac{\langle x-5 \rangle^2}{2} + C_1$$

$$EIY_1 = \frac{Rx^3}{12} - \frac{R\langle x-5 \rangle^3}{6} + C_1x + C_2$$

Para $x=0$ $y=0$ $C_2=0$

Para $x=10$ $y=0$

$$0 = \frac{R(10)^3}{12} - \frac{R(5)^3}{6} + 10C_1$$

$$C_1 = -\frac{25R}{4}$$

deflexion B \rightarrow Para $x=5$

$$EIY_B = \frac{R(5)^3}{12} - \frac{25R(5)}{4}$$

$$Y_B = -\frac{125R}{6EI}$$

$$EIY_2'' = (1700-R)x - M_A + R\langle x-10 \rangle$$

$$EIY_2' = (1700-R) \frac{x^2}{2} - M_Ax + R \frac{\langle x-10 \rangle^2}{2} + C_3$$

$$EIY_2 = (1700-R) \frac{x^3}{6} - \frac{M_Ax^2}{2} + R \frac{\langle x-10 \rangle^3}{6} + C_3x + C_4$$

Para $x=0$ $y_2=0$ $y_2'=0$ $C_3=0$ $C_4=0$

deflexion B Para $x=10$

$$EIY_B = (1700-R) \frac{(10)^3}{6} - \frac{M_A(10)^2}{2}$$

$$EIY_B = \frac{850000}{6} - \frac{500R}{3} - 1275000 + 500R$$

$$Y_B = \frac{1000R}{3EI} - \frac{2975000}{3EI}$$

$$0 = \frac{1000R}{3EI} - \frac{2975000}{3EI} = -\frac{125R}{6EI}$$

$$\frac{2125R}{6} = \frac{2975000}{3}$$

$$R = 2800 \text{ lb}$$

1700

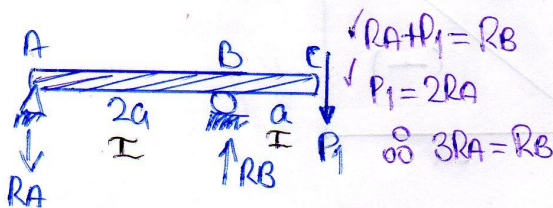
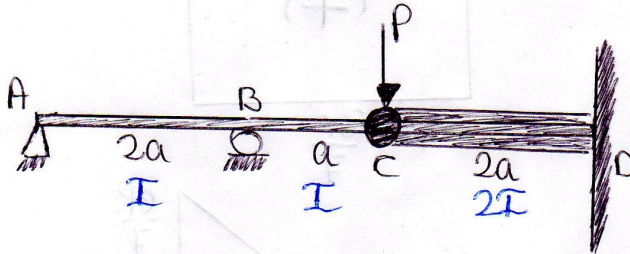


8500



Singularidad

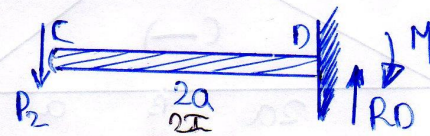
La viga ABCD mostrada, tiene una rótula en C y está empotrada en D. Utilizando el método de funciones de singularidad, dibujar los diagramas de fuerza cortante y momento flector.



$$\checkmark R_A + P_1 = R_B$$

$$\checkmark P_1 = 2R_A$$

$$\infty 3R_A = R_B$$



$$EI y_1'' = -R_A x + R_B \langle x - 2a \rangle$$

$$EI y_1' = -\frac{R_A x^2}{2} + \frac{R_B}{2} \langle x - 2a \rangle^2 + C_1$$

$$EI y_1 = -\frac{R_A x^3}{6} + \frac{R_B}{6} \langle x - 2a \rangle^3 + C_1 x + C_2$$

Para $x=0$ $y_1=0$ $C_2=0$

Para $x=2a$ $y_1=0$

$$0 = -\frac{R_A (2a)^3}{6} + C_1 (2a)$$

$C_1 = \frac{2R_A}{3} a^2$

$$2EI y_2'' = -P_2 x$$

$$2EI y_2' = -\frac{P_2 x^2}{2} + C_3$$

$$2EI y_2 = -\frac{P_2 x^3}{6} + C_3 x + C_4$$

Para $x=2a$ $y_2'=0$

Para $x=2a$ $y_2=0$

$C_3 = +2P_2 a^2$
 $C_4 = -\frac{8P_2}{3} a^3$

Para $x_1=3a$ $x_2=0$ $y_1=y_2$

$$-\frac{27R_A}{6} a^3 + \frac{R_B}{6} a^3 + 2R_A a^3 = -\frac{8P_2}{6} a^3$$

$$-\frac{5}{2} R_A + \frac{R_B}{6} + \frac{8P_2}{6} = 0$$

$$-2R_A = -\frac{8P_2}{6}$$

$\rightarrow R_A = \frac{2P_2}{3}$

$$\infty P_1 + P_2 = P$$

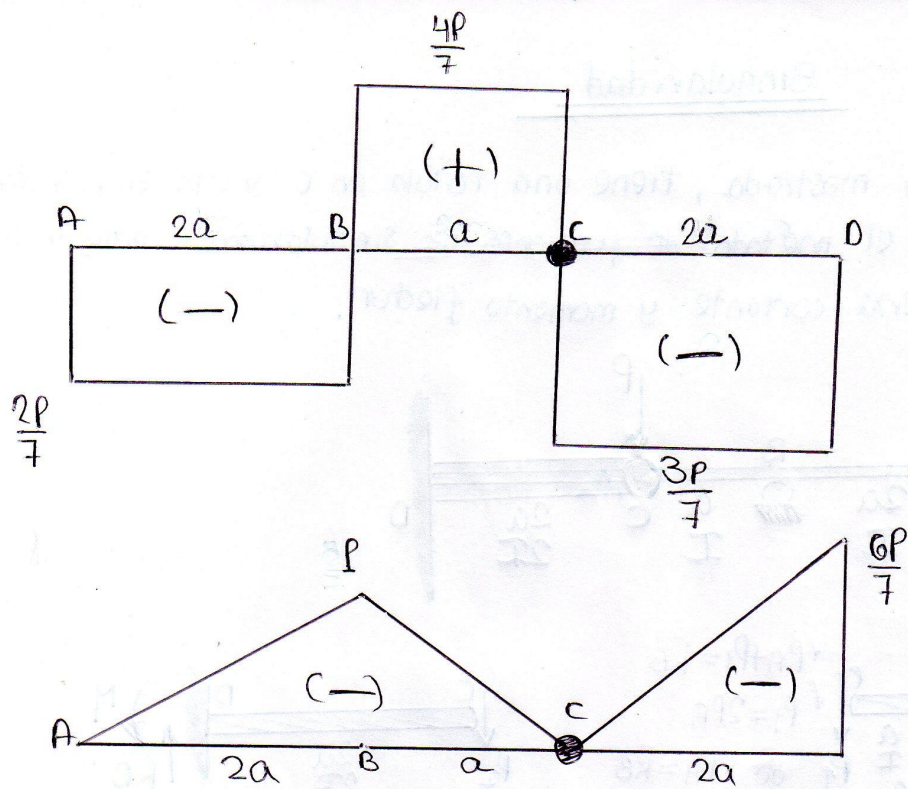
$$2R_A + \frac{3R_A}{2} = P$$

$$\frac{7R_A}{2} = P$$

$$\checkmark P_2 = R_D$$

$$\checkmark M = P_2 (2a)$$

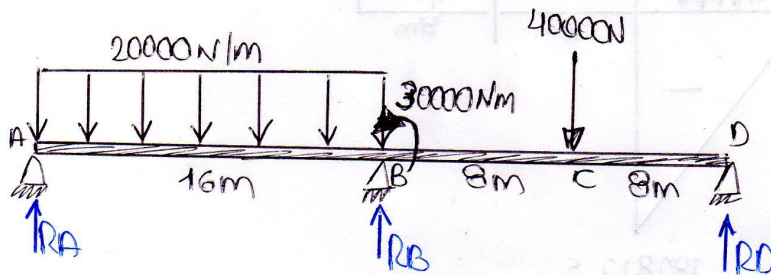
$R_A = \frac{2P}{7} (\downarrow)$	$R_B = \frac{5P}{7} (\uparrow)$	$R_D = \frac{3P}{7} (\uparrow)$	$M = \frac{6P}{7} (\downarrow)$
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$\frac{P}{7} = M$	$\frac{P}{7} = M$	$\frac{P}{7} = M$	$\frac{P}{7} = M$
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SINGULARIDAD

La viga mostrada Tiene $EI = cte$. Dibujar el diagrama de fuerza cortante y momento flector



$$EIy'' = R_D x - 40000 \langle x-8 \rangle + 30000 \langle x-16 \rangle^0 + R_B \langle x-16 \rangle - 20000 \frac{\langle x-16 \rangle^2}{2}$$

$$EIy' = \frac{R_D x^2}{2} - 20000 \langle x-8 \rangle^2 + 30000 \langle x-16 \rangle + \frac{R_B \langle x-16 \rangle^2}{2} - \frac{10000 \langle x-16 \rangle^3}{3} + C_1$$

$$EIy = \frac{R_D x^3}{6} - \frac{20000 \langle x-8 \rangle^3}{3} + 15000 \langle x-16 \rangle^2 + \frac{R_B \langle x-16 \rangle^3}{6} - \frac{2500 \langle x-16 \rangle^4}{3} + C_1 x + C_2$$

Para $x=16m$ $y=0$

$$0 = \frac{2048 R_D}{3} - \frac{10240000}{3} + 16 C_1$$

$$C_1 = \frac{640000}{3} - \frac{128 R_D}{3}$$

Para $x=32$ $y=0$

$$0 = \frac{16384 R_D}{3} - 92160000 + 3840000 + \frac{2048 R_D}{3} - \frac{163840000}{3} + 32 C_1$$

$$0 = \frac{16384 R_D}{3} + \frac{2048 R_D}{3} - 142933333.3 + 6826666.67 - \frac{4096 R_D}{3}$$

$$0 = 4096 R_D + \frac{2048 R_D}{3} - 136106666.7$$

ESTÁTICA

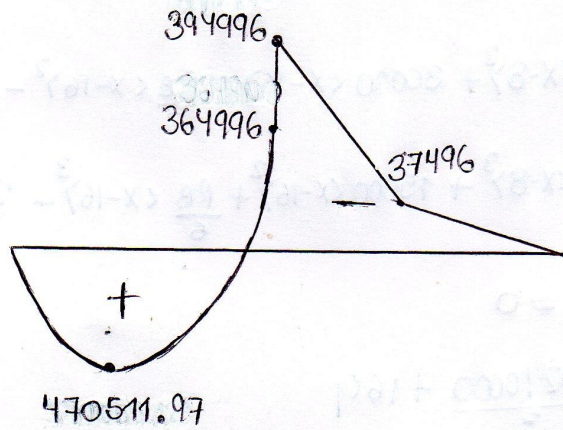
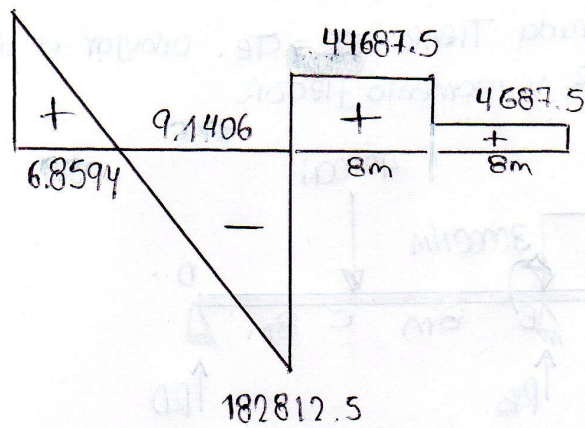
$$32 R_D + 16 R_B - 3490000 = 0$$

$$R_D = -4687.5$$

$$R_B = +227500$$

$$R_A = +137187.5$$

137187.5



$$\frac{dM}{dx} = \frac{dV}{dx} = 0$$

$$0 = 137187.5 - 182812.5x$$

$$137187.5 - 182812.5x = 0$$

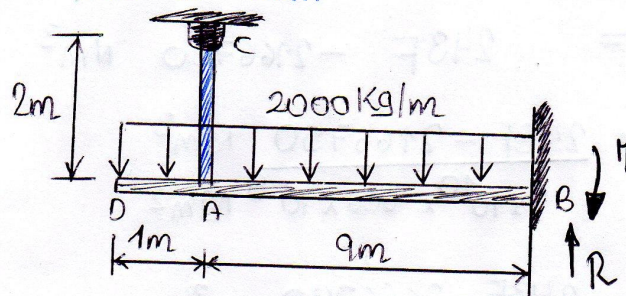
$$137187.5 = 182812.5x$$

$$x = \frac{137187.5}{182812.5}$$

$$x = 0.75$$

Singularidad

Para el sistema mostrado, formado por una viga (DAB) y un cable (CA) ambos de acero ($E = 2 \times 10^6 \text{ Kg/cm}^2$). se solicita dibujar los diagramas de fuerza cortante y momento flector de la viga DAB, sabiendo que para la viga $I = 500 \text{ cm}^4$ y para el cable $A = 5 \text{ cm}^2$



$$IEY'' = -\frac{2000x^2}{2} + F(x-1)$$

$$IEY' = -\frac{2000x^3}{6} + \frac{F(x-1)^2}{2} + C_1$$

$$IEY = -\frac{2000x^4}{24} + \frac{F(x-1)^3}{6} + C_1x + C_2$$

Para $x=10 \quad y'=0$

$$C_1 = \frac{2000(10^3)}{6} - \frac{F(9)^2}{2}$$

$$C_1 = \frac{1000000}{3} - \frac{81F}{2}$$

Para $x=10 \quad y=0$

$$0 = C_2 = \frac{2000(10)^4}{24} - \frac{F(9)^3}{6} - \frac{1000000 \times 10}{3} + \frac{810F}{2}$$

$$C_2 = \frac{567F}{2} - 2500000$$

✓ deflexion en la mga

Para $x_1 = y = ?$

$$IEy = -\frac{2000}{24} + \frac{10^6}{3} - \frac{81F}{2} + \frac{567F}{2} - 2500000$$

$$IEy = 243F - 2166750 \text{ m}^3$$

$$y = \frac{243F - 2166750 \text{ Kg m}^3}{2 \times 10^{10} \times 500 \times 10^{-8} \text{ Kg m}^2}$$

$$y = \frac{243F - 2166750}{100000} \text{ m}$$

✓ deformacion en la barra

$$\delta = \frac{FxL}{5 \times 10^{-4} \times 2 \times 10^{10}} = 2Fx \times 10^{-7}$$

defer = defle

$$2Fx \times 10^{-7} = \frac{243F - 2166750}{10^5}$$

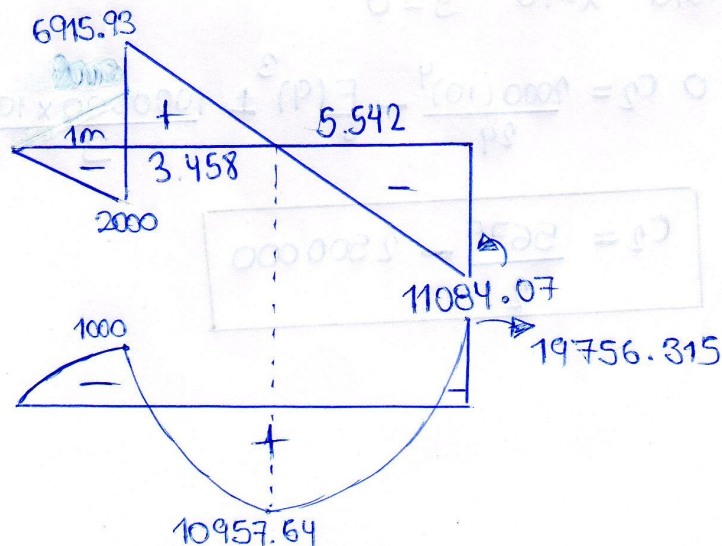
$$0.02F = 243F - 2166750$$

$$2166750 = 243.02F$$

$$F = 8915.93 \text{ Kg}$$

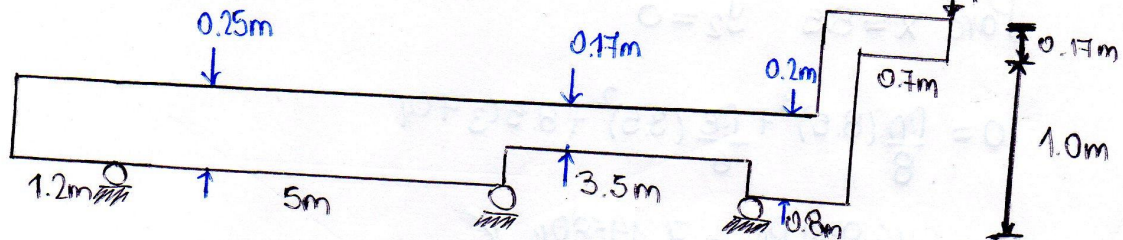
$$M = 19756.63$$

$$R_A = 11034.07 \text{ Kg}$$



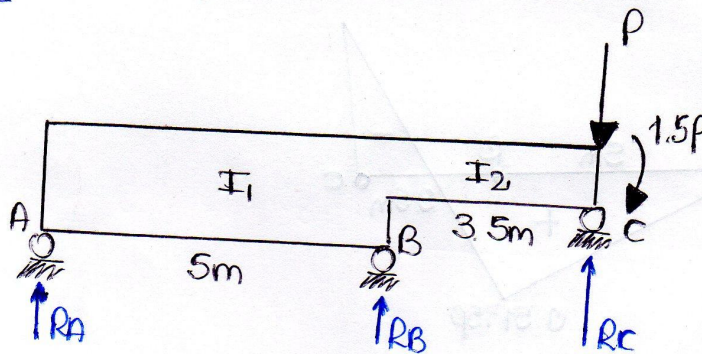
SINGULARIDAD

En la viga mostrada, se pide determinar el valor que debe tener la fuerza puntual "P", aplicada en el extremo del volado, de manera que en el apoyo "B" se genere un momento flector positivo de 3000 m-Kg.



$$I_1 = \frac{1m \times 0.25^3}{12} = 1.302 \times 10^{-3} m^4$$

$$I_2 = \frac{1m \times 0.17^3}{12} = 4.094 \times 10^{-4} m^4$$



$$I_1 E y_1'' = R_A x$$

$$I_1 E y_1' = R_A x^2/2 + C_1$$

$$I_1 E y_1 = R_A x^3/6 + C_1 x + C_2$$

Para $x=5$ $y_1=0$

$$0 = \frac{R_A x^3}{6} + C_1 x$$

$$\boxed{-\frac{25R_A}{6} = C_1}$$

$$I_2 E y_2'' = R_A x + R_B < x-5 >$$

$$I_2 E y_2' = R_A x^2/2 + R_B < x-5 >^2/2 + C_3$$

$$I_2 E y_2 = R_A x^3/6 + R_B < x-5 >^3/6 + C_3 x + C_4$$

Para $x=5$ $y_2=0$

$$0 = \frac{R_A x^3}{6} + C_3 x + C_4$$

$$\boxed{C_4 = 28.562 R_A}$$

Para $x=5$ $y_1' = y_2'$

$$\frac{R_A x^2}{2} + C_1 = \frac{I_1}{I_2} \left(\frac{R_A x^2}{2} + C_3 \right)$$

$$\frac{25R_A}{2} - \frac{25R_A}{6} = 3.18 \left(\frac{25R_A}{2} \right) + 3.18 C_3$$

$$\boxed{C_3 = -9.879 R_A}$$

• De la estática

$$R_A + R_B + R_C - P = 0 \quad \checkmark$$

$$0.8 R_A + 5 R_B + 8.5 R_C - 10 P = 0 \quad \checkmark$$

Para $x = 8.5 \quad y_2 = 0$

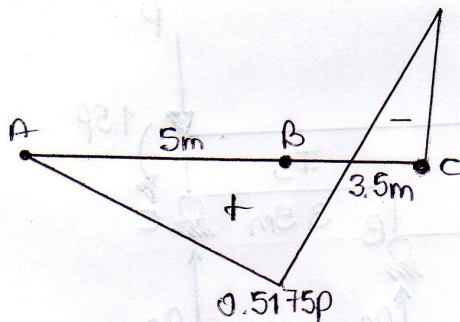
$$0 = \frac{R_A}{8} (8.5)^3 + \frac{R_B}{6} (3.5)^2 + 8.5 (3 + 4)$$

$$0 = 46.944 R_A + 7.1458 R_B \quad \checkmark$$

$$R_A = 0.1035 P$$

$$R_B = -0.6799 P$$

$$R_C = 1.5764 P$$

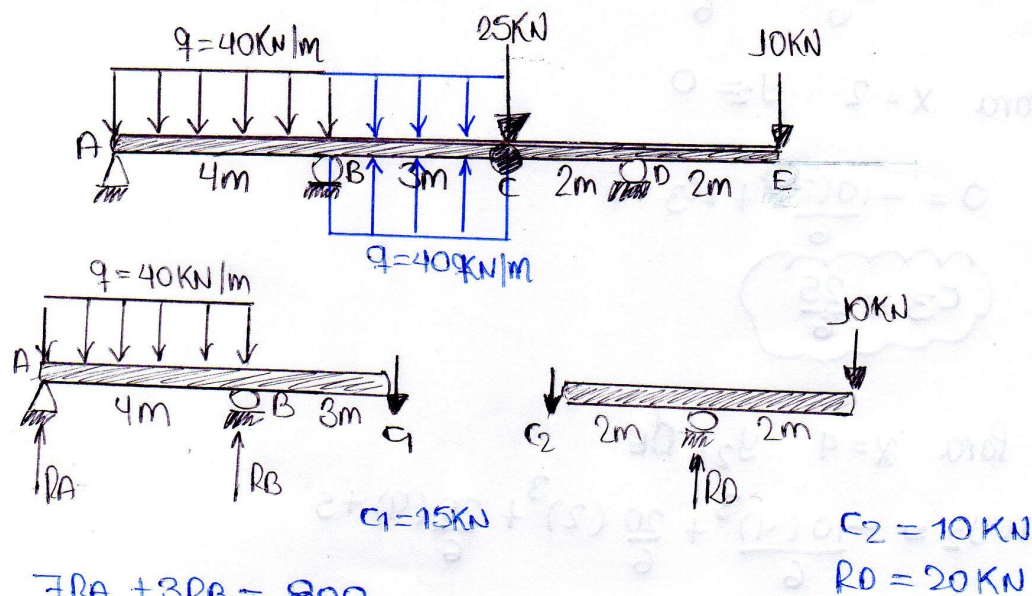


$$0.5175 P = 3000 \text{ m-Kg}$$

$$P = 5797.10 \text{ Kg}$$

Singularidad

DETERMINAR la deflexión en el punto E de la viga mastrada



$$7R_A + 3R_B = 800$$

$$R_A + R_B = 175$$

$$R_A = 68.75 \text{ kN}$$

$$R_B = 106.25 \text{ kN}$$

$$IEY''' = 68.75x - \frac{40}{2}x^2 + 106.25\langle x-4 \rangle + \frac{40}{2}\langle x-4 \rangle^2$$

$$IEY'' = 68.75x^2 - \frac{40x^3}{6} + \frac{106.25}{2}\langle x-4 \rangle^2 + \frac{40}{6}\langle x-4 \rangle^3 + C_1$$

$$IEY' = \frac{68.75x^3}{6} - \frac{40x^4}{24} + \frac{106.25}{6}\langle x-4 \rangle^3 + \frac{40}{24}\langle x-4 \rangle^4 + C_1x + C_2$$

Para $x=0$ $y_1=0$ $C_2=0$

Para $x_1=4$ $y_1=0$

$$0 = \frac{68.75}{6}(4)^3 - \frac{40(4)^4}{24} + 4C_1$$

$$C_1 = -\frac{230}{3}$$

Para $x=7$ $y_1=?$

$$EIY = \frac{68.75}{6}(7)^3 - \frac{40(7)^4}{24} + \frac{106.25}{6}(3)^3 + \frac{40}{24}(3)^4 + 7C_1$$

$$y = \frac{5}{EI}$$

$$IEY_2'' = -10\langle x \rangle + 20\langle x-2 \rangle$$

$$IEY_2' = -\frac{10x^2}{2} + \frac{20}{2}\langle x-2 \rangle^2 + C_3$$

$$IEY_2 = -\frac{10x^3}{6} + \frac{20}{6}\langle x-2 \rangle^3 + C_3x + C_4$$

Para $x_2=0$ $y_2=?$

$$IEY_2 = C_4$$

$$y_2 = \frac{C_4}{EI}$$

00 $y_2 = y_1$

$$\frac{C_4}{EI} = \frac{5}{EI}$$

$$C_4 = 5$$

$$IEY_2 = -\frac{10x^3}{6} + \frac{20}{6} \langle x-2 \rangle^3 + C_3x + 5$$

Para $x=2$ $y_2 = 0$

$$0 = -\frac{10(2)^3}{6} + 2C_3 + 5$$

$$C_3 = \frac{25}{6}$$

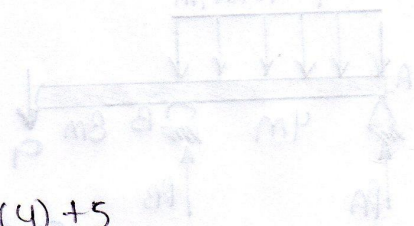
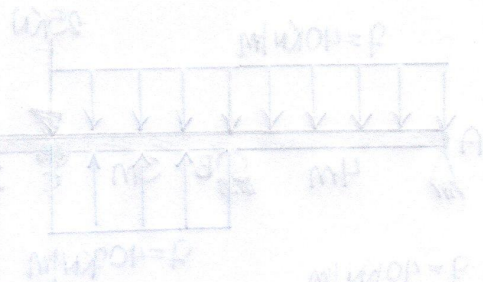
Para $x=4$ $y_2 = \delta_E$

$$IEY_2 = -\frac{10(4)^3}{6} + \frac{20}{6} (2)^3 + \frac{25}{6} (4) + 5$$

$$IEY_2 = -\frac{175}{3}$$

$$\delta_E = \frac{175}{3EI} \quad (\downarrow)$$

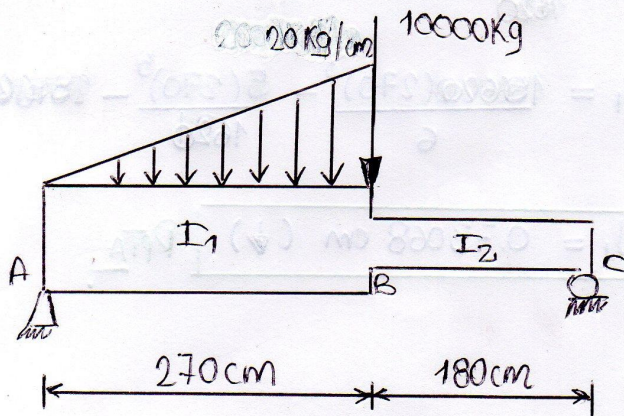
Rpta.



FUNCION DE SINGULARIDAD

Dada la viga simplemente apoyada de momento de inercia variable, determine el giro y la flecha en el punto de la carga aplicada de 10t/mf por el método de funciones de singularidad.

Se conoce $I_1 = 16650 \text{ cm}^4$ $I_2 = 12500 \text{ cm}^4$ $E = 2 \times 10^6 \text{ Kg/cm}^2$



ESTÁTICA

$$450R_c = 10000(270) + \frac{1}{2}(270)(270) \times \frac{20}{3}$$

$$R_c = 7080 \text{ Kg } (\uparrow)$$

$$R_A = 5620 \text{ Kg } (\uparrow)$$

$$I_1 E y_1''' = R_A x_1 - \frac{20}{270} x_1 \left(\frac{x_1}{2} \right) \left(\frac{x_1}{3} \right)$$

$$I_1 E y_1'' = R_A x_1 - \frac{1}{81} x_1^3$$

$$I_1 E y_1' = \frac{R_A x_1^2}{2} - \frac{1}{324} x_1^4 + C_1$$

$$I_1 E y_1 = \frac{R_A x_1^3}{6} - \frac{1}{1620} x_1^5 + C_1 x_1 + C_2$$

$$I_2 E y_2'' = R_c x_2$$

$$I_2 E y_2' = \frac{R_c x_2^2}{2} + C_3$$

$$I_2 E y_2 = \frac{R_c x_2^3}{6} + C_3 x_2 + C_4$$

Para $x_1 = 270$ $x_2 = 180$ $y_1 = y_2$

$$\frac{5620(270)^3}{6} - \frac{1(270)^5}{1620} + 270C_1 = \left[\frac{7080(180)^3}{6} + 180C_3 \right] \times \frac{16650}{12500}$$

$$270C_1 - 239.76C_3 + 8384170680 = 0$$

Para $x_1 = 270$ $x_2 = 180$ $y_1' = -y_2'$

$$\frac{5620(270)^2}{2} - \frac{1(270)^4}{324} + C_1 = - \left[\frac{7080(180)^2}{2} + C_3 \right] \times \frac{16650}{12500}$$

$$C_1 + 1.332C_3 + 341221572 = 0$$

$$C_1 = -155120119.2$$

$$C_3 = -139715805.4$$

Hallando deflexión

$$EI_y y_1 = \frac{R_A x^3}{6} - \frac{1 x_1^5}{1620} + C_1 x_1$$

$$16650(2 \times 10^6) y_1 = \frac{5620(270)^3}{6} - \frac{(270)^5}{1620} - 155120119.2(270)$$

$$y_1 = 0.73068 \text{ cm } (\downarrow)$$

Rpta

Hallando giro

$$EI_y y_1' = \frac{R_A x^2}{2} - \frac{x^4}{324} + C_1$$

$$16650 \times 2 \times 10^6 y_1' = \frac{5620(270)^2}{2} - \frac{270^4}{324} - 155120119.2$$

$$y_1' = 1.00 \times 10^{-3} \text{ rad - antihorario}$$

Rpta